AFGHAN NATIONAL POLICE STANDARD BUILDING DESIGNS

GUARD TOWER

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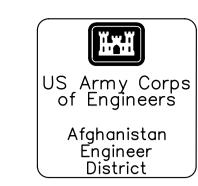
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				APR	
				DATE	
				DESCRIPTION	
				SYMBOL	,

DESIGNED BY: DATE: JDS DWN BY: SUBMI JDS CHK BY: FILE N JDS AN

AFGHAN NATIONAL POLICE STANDARD DESIGN GUARD TOWER COVER SHEET

SHEET REFERENCE NUMBER: **G1** GENERAL NOTES

1.0 THIS PROJECT HAS BEEN DESIGNED FOR THE WEIGHTS AND MATERIALS INDICATED ON THE SHEETS AND FOR THE LIVE LOADS INDICATED IN THE DESIGN DATA. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ALLOWABLE CONSTRUCTION LOADS AND TO PROVIDE PROPER DESIGN AND CONSTRUCTION OF FALSEWORK, FORMWORK, STAGING, BRACING, SHEETING AND SHORING, ETC.

1.1 COORDINATE THESE SHEETS WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND CIVIL SHEETS. ALL DIMENSIONS SHOWN ON THE SHEETS ARE MILLIMETERS UNLESS NOTED OTHERWISE.

- 1.2 THE CONTRACTOR SHALL VERIFY AND COORDINATE ALL FLOOR AND ROOF OPENING SIZES AND LOCATIONS, EQUIPMENT PAD SIZES AND LOCATIONS, ANCHOR BOLT LAYOUTS, ETC WITH EQUIPMENT SELECTED. THE CONTRACTOR SHALL MAKE ANY REQUIRED MODIFICATIONS AT NO ADDITIONAL COST
- 1.3 THE CONTRACTOR SHALL REFER TO THE ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING SHEETS FOR SLEEVES, CURBS, INSERTS OR OPENINGS, ETC. NOT HEREIN INDICATED.
- 1.4 SLAB OPENINGS SMALLER THAN 250mm DIA TO BE CORE DRILLED IN FIELD UON. SEE MECHANICAL, ELECTRICAL AND PLUMBING SHEETS FOR LOCATIONS OF THESE OPENINGS.
- 1.5 WORK NOT INCLUDED ON THE SHEETS BUT IMPLIED TO BE SIMILAR TO THAT SHOWN AT CORRESPONDING PLACES ELSEWHERE ON THE SHEETS SHALL BE REPEATED
- 1.6 IN CASE OF CONFLICT BETWEEN THE NOTES, DETAILS AND SPECIFICATIONS THE MOST RIGID REQUIREMENTS SHALL GOVERN 1.7 SEE ARCHITECTURAL SHEETS FOR LOCATIONS OF MASONRY AND DRYWALL NON-LOAD
- BEARING PARTITIONS. PROVIDE COMPRESSIBLE FIRESAFING AT TOP OF WALL AS REQUIRED BY ARCHITECTURAL SHEETS. 1.8 COORDINATE FINISHED FLOOR DATUM ELEVATION 0.0m WITH THE CIVIL SHEETS.

2.0 <u>FOUNDATION NOTES</u>

- THE GEOTECHNICAL ANALYSIS FOR THIS PROJECT IS THE RESPONSIBILITY OF THE CONTRACTOR AWARDED THE WORK. DESIGN VALUES USED IN THE STRUCTURAL ANALYSIS OF THE BUILDINGS HEREIN INDICATED HAVE BEEN ASSUMED AND SHALL BE CONFIRMED AND VERIFIED AS PART OF THE GEOTECHNICAL INVESTIGATION. VALUES WHICH DO NOT MEET THE REQUIREMENTS INDICATED ON SHEET S2 SHALL BE IMMEDIATELY BROUGHT TO THE ATTENTION OF THE CONTRACTING OFFICER FOR CONSIDERATION AND DETERMINATION ON THE NEXT APPROPRIATE COURSE OF ACTION.
- 2.2 SEE THE SPECIFICATION FOR ADDITIONAL REQUIREMENTS TO THOSE OUTLINED IN THE GEOTECHNICAL INVESTIGATION FOR EXCAVATION AND PREPARATION OF THE FOUNDATION AND THE SLAB ON GRADE SUBGRADE INCLUDING COMPACTION PROCEDURES.
- 2.3 EXCAVATIONS FOR FOOTINGS SHALL HAVE THE SIDES AND BOTTOMS TEMPORARILY LINED WITH 0.25mm POLYETHYLENE IF PLACEMENT OF CONCRETE DOES NOT OCCUR WITHIN 24 HRS OF THE EXCAVATION OF THE FOOTING.
- 2.4 FOUNDATION CONDITIONS NOTED DURING CONSTRUCTION WHICH DIFFER FROM THOSE DESCRIBED IN THE GEOTECHNICAL REPORT SHALL BE REPORTED TO THE GENERAL CONTRACTOR BEFORE FURTHER CONSTRUCTION IS ATTEMPTED. SEE PROJECT SPECIFICATIONS.
- 2.5 NO FOOTINGS OR SLABS SHALL BE POURED INTO OR AGAINST SUBGRADE CONTAINING FREE WATER, FROST, ICE OR LOOSE MATERIAL. FROST DEPTH ASSUMED TO BE 800MM
- 2.6 ALL SLAB-ON-GRADE, TRENCH BOTTOMS AND OTHER ON-GRADE INTERIOR HORIZONTAL SURFACES SHALL BE PLACED OVER A 0.25mm VAPOR RETARDER OVER A 100mm #57 STONE WATER BARRIER PLACED ON SUBGRADE PROPERLY PREPARED IN ACCORDANCE WITH THE CONTRACT SPECIFICATIONS. (UON)
- 2.7 SEE PLUMBING, ELECTRICAL & CIVIL SHEETS FOR REQUIRED UNDERSLAB UTILITIES. 2.8 SEE ARCHITECTURAL SHEETS FOR ALL WATERPROOFING DETAILS AND MATERIALS. 2.9 IF UNDERMINING OF FOOTINGS OCCURS, FILL VOIDS WITH 15MPa CONCRETE. DO NOT ATTEMPT TO REPLACE AND RECOMPACT SOIL.

3.0 CONCRETE

- 3.1 CONCRETE SHALL HAVE THE UNIT WEIGHT AND THE MINIMUM COMPRESSIVE STRENGTHS (f'c) AT 28 DAYS AS SHOWN IN THE CONCRETE MATERIALS SCHEDULE ON THIS SHEET. SEE SPECIFICATIONS FOR ADDITIONAL INFORMATION. ENTRAIN AIR TO PRODUCE TOTAL AIR CONTENT ACCORDING TO THE SPECIFICATIONS FOR CONCRETE EXPOSED TO FREEZING TEMPERATURES (EXTERIOR FOOTINGS, SLAB TURNDOWNS, EXTERIOR SLABS AND SLABS-ON-GRADE, EXTERIOR RETAINING WALLS, AND EXTERIOR GRADE BEAMS.)
- MWFRS MAIN WIND FORCE RESISTING SYSTEM 3.2 GROUT FOR BASE PLATES SHALL BE NON-SHRINKABLE GROUT AND SHALL HAVE A MINIMUM SPECIFIED COMPRESSIVE STRENGTH AT 28 DAYS OF 35MPa. UNLESS NOTED
 - 3.3 NO CALCIUM CHLORIDE SHALL BE USED IN ANY CONCRETE 3.4 MIXING. TRANSPORTING AND PLACING OF CONCRETE SHALL CONFORM TO
 - 301M-05.
 - 3.5 ALL CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF THE AMERICAN CONCRETE INSTITUTE (ACI) 318M MANUAL (metric), "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE". AND REQUIREMENTS OUTLINED IN THE CONTRACT SPECIFICATIONS. WHEN THERE IS A CONFLICT BETWEEN ACI AND THE SPECIFICATIONS. THE MORE STRINGENT SHALL GOVERN.
 - 3.6 CHAMFER ALL EXPOSED EXTERNAL CORNERS OF CONCRETE WITH 20mm x45 DEGREE CHAMFER UON.
 - 3.7 CONCRETE REINFORCEMENT BARS SHALL CONFORM TO ASTM A615M, GRADE 420, REINFORCING BARS SHALL NOT BE TACK WELDED, WELDED, HEATED OR CUT, UNLESS INDICATED ON THE CONTRACT DOCUMENTS. ALL LAP SPLICES SHALL BE CLASS "B"
 - 3.8 HORIZONTAL FOOTING AND HORIZONTAL WALL REINFORCEMENT SHALL BE CONTINUOUS AND SHALL HAVE 90 DEGREE BENDS AND EXTENSIONS. OR CORNER BARS OF EQUIVALENT SIZE LAPPED WITH A CLASS B TENSION SPLICE AT CORNERS AND INTERSECTIONS. TOP BAR CRITERIA SHALL APPLY IF 300mm OR MORE OF FRESH CONCRETE IS PLACED BELOW BAR.
 - 3.9 SLABS-ON-GRADE SHALL HAVE CONSTRUCTION JOINTS OR CRACK CONTROL JOINTS AS SHOWN ON THE SHEETS. CONSTRUCTION JOINTS CAN BE USED AT CONTROL JOINT LOCATIONS AT CONTRACTORS OPTION. SEE SLAB PLANS & JOINT DETAILS FOR ADDITIONAL INFORMATION. FOR AREAS NOT SHOWN ON THE SHEETS, THE MAXIMUM SPACING OF CONSTRUCTION/ CRACK CONTROL JOINTS SHALL BE 4800 mm.
 - 3.10 SEE SPECIFICATIONS FOR ALL WATERPROOFING/DAMPPROOFING REQUIREMENT. 3.11 ALL CONCRETE REINFORCEMENT SHALL BE DETAILED, FABRICATED, LABELED, SUPPORTED, AND SPACED IN FORMS AND SECURED IN PLACE IN ACCORDANCE WITH THE PROCEDURES AND REQUIREMENTS OUTLINED IN THE LATEST EDITION OF THE "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE", ACI 318M, AND THE "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES". ACI 315M, LATEST EDITION.

- 3.12 SHOP DRAWINGS SHOWING REINFORCING DETAILS, INCLUDING STEEL SIZES, SPACING AND PLACEMENT, SHALL BE SUBMITTED FOR REVIEW PRIOR TO FABRICATION.
- 3.13 ALL DOWELS SHALL MATCH SIZE AND NUMBER OF MAIN REINFORCING, UNLESS NOTED
- OTHERWISE ON THE SHEETS 3.14 ADDITIONAL BARS SHALL BE PROVIDED AROUND ALL FLOOR AND WALL OPENINGS AS SHOWN ON THE SHEETS
- 3.15 SEE ARCHITECTURAL SHEETS FOR TYPE AND LOCATION OF ALL FLOOR FINISHES
- 3.16 THE CONTRACTOR SHALL COORDINATE ADDITIONAL WALL/SLAB OPENINGS NOT SHOWN ON STRUCTURAL SHEETS. SEE MECHANICAL. ELECTRICAL. PLUMBING AND CIVIL SHEETS. 3.17 THE SUB-CONTRACTOR SHALL VERIFY ALL OPENINGS, PAD SIZES, AND ANCHOR BOLTS
- WITH EQUIPMENT SELECTED 3.18 FOR ALL WALLS & PIERS, PROVIDE DOWELS INTO FOOTING AT EACH VERT REINF BAR
- UON DOWEL SIZE SHALL BE SAME AS VERT REINF. 3.19 ALL REINFORCING INDICATED TO BE WELDED SHALL BE IN ACCORDANCE WITH ASTM A706M. "LOW ALLOY STEEL DEFORMED BARS FOR CONCRETE REINFORCEMENT". ANY INSTALLATIONS USING MANUFACTURER'S EQUIPMENT SHALL BE PER MANUFACTURER'S
- RECOMMENDATIONS. 3.20 PROVIDE CONCRETE POUR STOPS OR FORMED AS REQUIRED FOR INSTALLATION OF ALL CONCRETE WORK.
- 3.21 PROVIDE ADDITIONAL $(2)-\#13 \times 600$ mm REINFORCING BARS IN SLAB-ON GRADE AT ALL RE-ENTRANT CORNERS. PLACE BARS AT MID-DEPTH OF SLAB WITH A CLEARANCE OF 50mm FROM CORNER UON.

4.0 CONCRETE MASONRY

- MASONRY CONSTRUCTION AND MATERIALS SHALL CONFORM TO ALL REQUIREMENTS OF THESE CONTRACT DOCUMENTS AND THE PROJECT SPECIFICATIONS
- 4.2 THE SPECIFIED ULTIMATE COMPRESSIVE STRENGTH OF CONCRETE MASONRY (f'm) ON THE NET AREA IS A MINIMUM OF 10.4 MPa.
- 4.3 PROVIDE TWO #16 BARS CONTINUOUS IN ALL CMU AND CAST-IN-PLACE BOND BEAMS UON ON THE SHEETS. BOND BEAMS SHALL BE CONTINUOUS AND SPACED AT A MAXIMUM OF 1200mm OC VERTICALLY. PROVIDE BOND BEAM STARTER COURSE AT BOTTOM OR FIRST COURSE ON ALL MASONRY WALLS AND PARTITIONS. ALL BOND BEAMS SHALL BE A MINIMUM OF 200mm IN DEPTH WITH REINFORCING BEING CONTINUOUS AND HAVING STANDARD ACI 180° HOOKS AT EACH END. PROVIDE STANDARD BAR SPLICES AS SPECIFIED.
- 4.4 FOR WALL REINFORCING, SEE DETAIL 7 ON SHEET S5
- 4.5 CMU CELLS THAT REQUIRE VERTICAL REINFORCING BARS AS INDICATED ON THE CONTRACT DRAWINGS AND/OR SPECS SHALL HAVE REINF BARS PLACED IN CENTERS OF CMU CELLS AND CONTINUOUSLY GROUTED UON.
- 4.6 PROVIDE LADDER TYPE JOINT REINFORCEMENT AT (200mm EXTERIOR & 400mm INTERIOR) ON CENTER MAXIMUM UON MINIMUM ROD SIZE USED SHALL BE 9 GA. DEFORMED WIRE AND CONFORM TO ASTM A82M, UON.
- 4.7 PROVIDE CONTROL JOINTS AS INDICATED ON THE ARCHITECTURAL SHEETS.
- 4.8 GROUT FOR MASONRY SHALL BE NORMAL WEIGHT AND HAVE A MINIMUM COMPRESSIVE STRENGTH OF 25 MPa AT 28 DAYS. GROUT SHALL CONFORM TO ASTM C476M. GROUT LIFTS SHALL NOT EXCEED 1400mm
- 4.9 USE MORTAR TYPE S CONFORMING TO ASTM C270M, SEE SPECIFICATIONS.
- 4.10 CONCRETE MASONRY UNITS SHALL BE NORMAL WEIGHT AND CONFORM TO ASTM C90M. 4.11 ALL CMU CELLS, OPEN CAVITIES, AND AIR SPACES SHALL BE GROUTED TO STOP
- FRAGMENTS FROM MORTAR BLAST 4.12 BOND BEAM REINFORCING SHALL BE DISCONTINUOUS AT CONTROL JOINTS (UON) MAXIMUM CONTROL JOINT SPACING SHALL BE AS INDICATED ON THE ARCHITECTURAL SHEETS.
- 4.13 CONTRACTOR SHALL COORDINATE LOCATION OF ALL OPENINGS SEE ARCH, MECH, ELEC, AND PLUMBING SHEETS. FOR SIZE AND LOCATION OF OPENINGS.
- 4.14 MASONRY WALLS SHALL NOT BE BACK FILLED PRIOR TO THE MORTAR AND GROUT ATTAINING THEIR RESPECTIVE MAXIMUM DESIGN STRENGTHS PER SPECIFICATIONS.
- 5.0 CFMRF COLD FORM METAL ROOF FRAMING SYSTEM
- 5.1 CFMF SHALL BE DESIGNED BY CFMF MANUFACTURER'S ENGINEER FOR ALL LOADING
- PER CODE AND AS INDICATED ON THE SHEETS. 5.2 FOR WIND LOADS, SEE THE DESIGN CRITERIA ON SHEET S2.
- 5.3 SUBMIT VENDOR'S PUBLISHED LITERATURE, TEST DATA AND INSTALLATION INSTRUCTIONS FOR METAL STUD ASSEMBLY AND ACCESSORIES INCLUDING OTHER DATA AS MAY BE REQUIRED TO CERTIFY COMPLIANCE WITH PERFORMANCE REQUIREMENTS SPECIFIED HEREIN.
- 5.4 SHOP DRAWINGS AND DESIGN ANALYSIS SHALL BE STAMPED AND APPROVED BY A LICENSED PROFESSIONAL ENGINEER.
- 5.5 CONNECTIONS AND GAUGE SIZES ARE MINIMUM AND SHALL BE INCREASED AS NECESSARY TO PROVIDE A STRUCTURALLY ADEQUATE SYSTEM. KICKERS MAY BE ADDED TO REDUCE THE STUD HEIGHTS WHERE ACCEPTABLE AND COORDINATED WITH THE ARCHITECTURAL DRAWINGS.
- 5.6 CRMRF SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES:
- STUD/RAFTER/EAVE STRUT/BRACE/BLOCKING: Fv = 344 MPaGAUGE = 18

DEPTH = 152.3 mmWIDTH = 34.8 mmMOMENT OF INERTIA, $Ix = 847 \times 10^3 \text{ mm}^4$ SECTION MODULUS, $Sx = 11.2x10^3 \text{ mm}^3$ TRACK:

 $F_V = 344 \text{ MPa}$ GAUGE = 16DEPTH = 152.3 mmWIDTH = 38 mmMOMENT OF INERTIA, $Ix = 1083 \times 10^3 \text{ mm}^4$ SECTION MODULUS, $Sx = 13.8 \times 10^3 \text{ mm}^3$ PURLIN/SUBGIRT:

Fy = 393 MPa

GAUGE = 16MOMENT OF INERTIA (TOP COMPRESSION), $Ixt = 23.7x10^3 \text{ mm}^4$

MOMENT OF INERTIA (BOTT COMPRESSION), $lxb = 22.7x10^3 \text{ mm}^4$ SECTION MODULUS (TOP COMPRESSION), Sxt = 1.8×10^3 mm³ SECTION MODULUS (BOTT COMPRESSION), $Sxb = 1.7x10^3 \text{ mm}^3$

MINIMUM LAP SPLICES OF REINFORCING BARS IN TENCION (DED ACI 319M OF)

IN TENSION (PER ACT 318M-05)							
f'c = 28 MPa CONCRETE							
	CENTER TO	(TOP E	BARS)	(OTHER BARS)			
BAR SIZE	CENTER BAR SPACING	LESS THAN 4db	4db OR MORE	LESS THAN 4db	4db OR MORE	4db	
	#10	460	460	410	410	40	
	#13	660	610	510	480	50	
	#16	1020	760	790	580	60	

NOTES:

- LAP SPLICES ABOVE ARE IN MILLIMETERS UON. YIELD STRENGTH OF REINFORCEMENT, fy, IS 420MPa (LAP SPLICE LENGTH IS IN MILLIMETERS). 6.
- CONCRETE IS NORMAL WEIGHT (2400Kg/m³)
- 4. TOP BAR INDICATES HORIZONTAL REINFORCEMENT WHICH IS PLACED ABOVE 300mm OR MORE OF FRESH CONCRETE.

STRUCTURAL

ELEMENT

SLAB-ON-GRADE/TURN-DOWN SLABS

ROOF AND FLOOR SLABS

ALL FOOTINGS (UON)

MISC. CURBS, WALLS

CAST-IN-PLACE LINTEL

AND PADS UON

CONCRETE MATERIALS SCHEDULE

ALL CONCRETE SHALL BE NORMAL WEIGHT

STANDARD HOOKS

IN TENSION PER

(ACI 318M - 05)

HOOK DEVELOPMENT LENGTH Ldh

(mm)

f'c 28 MPa

180

250

300

380

430

480

560

610

690

CONCRETE. (2400 Kg/m³ UON)

WATER-CEMENT RATIO OF 0.45.

ALL CONCRETE SHALL HAVE A

BAR SIZE

#10

#13

#16

#19

#22

#25

#29

#32

#36

CONCRETE

COMPRESSIVE

STRENGTH @

28 DAYS

(MPa)

28

28

28

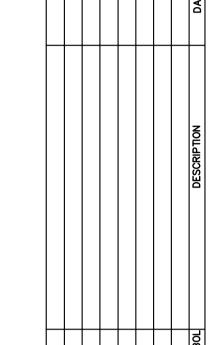
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- SEE COLUMN SCHEDULE FOR COLUMN AND SHEAR WALL VERTICAL LAP SPLICE.

ON CENTER CORRESPOND TO CRSI CATEGORY 5.

STRAIGHT DEVELOPMENT LENGTH OF AN UNLAPPED BAR IS EQUAL TO VALUE FROM TABLE DIVIDED BY 1.3. CATEGORY FOR BARS SPACED LESS THAN 4d, OR ON CENTER CORRESPONDS TO CATEGORY 1 IN THE CRSI

HANDBOOK WHEREAS FOR BARS SPACED 4d, OR MORE



DESIGNED
GDH
DWN BY:
MDB
CHK BY:
CWW

JS_A<u>r</u>my Corps

of Engineers

Afghanistan

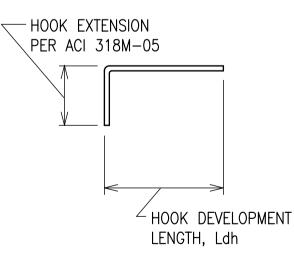
Engineer

District

	SONRY REINFORCING IMUM LAP SPLICES
BAR SIZE	BASIC LAP SPLICE Ld FOR CMU REINFORCING(mm)
#10	450
#13	600
#16	750

	ONRY REINFORCING IMUM LAP SPLICES
BAR SIZE	BASIC LAP SPLICE Ld FOR CMU REINFORCING(mm)
#10	450
#13	600
#16	750

HOOK EXTENSION PER ACI 318M-05 2. BAR YIELD STRENGTH, fy = 420 MPa ARE ASSUMED TO NOT BE MET.



- 1. CONCRETE IS NORMAL WEIGHT CONCRETE.
- 3. SIDE COVER REQUIREMENTS OF ACI SECT. 12.5.3
- 4. TIE OR STIRRUP REQUIREMENTS OF ACI SECT. 12.5.3 ARE ASSUMED TO NOT BE MET.
- 5. REDUCTION FOR EXCESS REINFORCEMENT IS NOT TAKEN.
- 6. HOOK DEVELOPMENT LENGTH IS VALID FOR 180° HOOKS ALSO.

REFERENCE NUMBER: **S1**

IAN NATIONAL STANDARD DESIGN

SHEET

NOTES

GENERAL

1.0 DESIGN LOADS

1.1 DEAD LOADS

1.1.1 ROOF DEAD LOADS — CONVENTIONAL FRAMING

	MAXIMUM GRAVITY LOAD	MINIMUM GRAVITY LOAD
LIGHT GAUAGE FRAMING METAL ROOFING INSULATION (batt) MISC	0.20 KPa 0.14 KPa 0.10 KPa 0.05 KPa 0.49 KPa	0.15 KPa 0.05 KPa 0.05 KPa 0.00 KPa 0.25 KPa

1.1.2 DOOE DEAD LOADS CONCRETE EDAMING

.1.2	ROOF	DEAD	LOADS	_	CONCRETE FRAMING
					MAXIMUM GRAVITY LOAD
	FLAT ELEC		BING		4.80 KPa 0.15 KPa 0.05 KPa
					5.00 KPa

1.2 LIVE LOADS (PER IBC 2006)

1.2.1 ROOF LIVE LOADS: ALL BUILDINGS

GREATER OF 1.0 KPa MINIMUM OR SNOW LOAD

1.2.2 ELEVATED FLOOR UNIFORM LIVE LOADS

GUARD TOWER 4.80 KPa

1.2.3 SLAB-ON-GRADE LIVE LOADS

4.80 KPa ALL BUILDINGS

1.3 SNOW LOADS (PER IBC 2006)

1.3.1 DESIGN PARAMETERS

SEISMIC BASE SHEAR

SIZE (mm)

(BxH)

400x400

SIZE (mm)

(BxH)

400x400

TOP

(2)-#25

TOP

(2)-#16

FLOOR BEAM

MARK

FB1

ROOF BEAM

MARK

RB1

GROUND SNOW LOAD (per UFC 3-310-01) PER LOCAL CONDITION 1.0 KPa SNOW IMPORTANCE FACTOR

1.0 KPa SNOW EXPOSURE FACTOR

1.4 SEISMIC LOADS (PER IBC 2006 & UFC 3-310-04)

1.4.1 SEISMIC PARAMETERS - LOAD BEARING MASONRY

SEISMIC OCCUPANCY CATEGORY	ll .
SEISMIC IMPORTANCE FACTOR (I)	1.0
SEISMIC SITE CLASS	D
Ss	1.280
S1	0.510
Sds	0.853
Sd1	0.510
SEISMIC DESIGN CATEGORY	D
SEISMIC RESISTING SYSTEM	MOMENT RESISTING FRAME SYSTEM
	SPECIAL REINF CONC MOMENT FRAME
RESPONSE MODIFICATION FACTOR (R)	8.0
RESPONSE COEFFICIENT (Cs)	0.133
SEISMIC ANALYTICAL PROCEDURE	EQUIV LATERAL FORCE

CONCRETE BEAM SCHEDULE

REINFORCING

REINFORCING

BOTTOM

(2)-#25

(2)-#16

STIRRUPS

#13 @ 250

STIRRUPS

#13 **@** 250

1.5 WIND LOADS (PER IBC 2006)

1.5.1 DESIGN PARAMETERS

BASIC WIND SPEED	137 Km/h
WIND IMPORTANCE FACTOR	1.0
WIND EXPOSURE CATEGORY	
DIRECTIONALITY COEFFICIENT (Kd)	0.85
TOPOGRAPHIC FACTOR (Kzt)	1.0

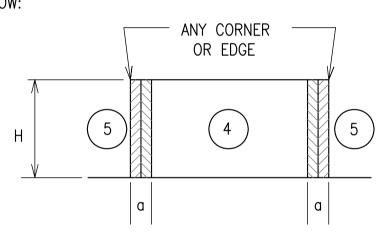
1.5.2 DESIGN WIND PRESSURE - MAIN WINDFORCE RESISTING SYSTEM

LOCATION	CORNER ZONE WIDTH "a"	MEAN ROOF HEIGHT "h"	WINDWARD WALL (@ MEAN ROOF HEIGHT)	LEEWARD WALL (@ MEAN ROOF HEIGHT)	ROOF
FIELD ZONE	N/A	6025mm	528 N/m²	298 N/m²	803 N/m²
CORNER ZONE	900mm	6025mm	800 N/m²	515 N/m ²	-1244 N/m ²

- a = 10% OF LEAST HORIZONTAL DIMENSION OR 0.4h, WHICHEVER IS SMALLER,
- BUT NOT LESS THAN EITHER 4% OF LEAST HORIZONTAL DIMENSION OR 0.9M. h = MEAN ROOF HIEGHT, IN METERS, EXCEPT THAT EAVE HIEGHT SHALL BE USED FOR ANGLE GREATER THAN 10°.

1.5.3 DESIGN WIND PRESSURE - WALL COMPONENTS AND CLADDING

EXTERIOR WALL SYSTEMS & THEIR ATTACHMENTS TO THE PRIMARY STRUCTURE SHALL BE DESIGNED FOR THE PRESSURES SHOWN IN THE DIAGRAM BELOW:



LOCATION	WINDWARD N/m ² (PRESSURE (inward)	LEEWARD N/m² (a	
	4	5	4	5	
MAIN BUILDING					(mm)
$AREA = 1 m^2$	943	943	-1020	-1264	900
$AREA = 2 m^2$	900	900	-982	-1173	900
$AREA = 5 m^2$	804	804	-886	-981	900
$AREA = 10 m^2$	804	804	-886	-981	900

REMARKS

TOP BAR LAP AT CENTER

BOTT BAR LAP PAST COL

REMARKS

TOP BAR LAP AT CENTER

BOTT BAR LAP PAST COL

75.6 kN

MIDBAR

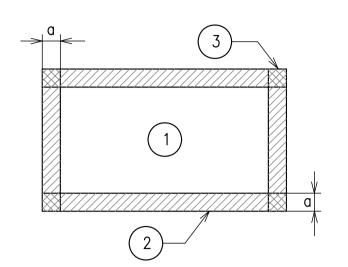
MIDBAR

N/A

- 1. DESIGN WIND PRESSURES ABOVE REPRESENT THE NET PRESSURE (SUM OF INTERNAL AND EXTERNAL PRESSURE) APPLIED NORMAL TO ALL SURFACES.
- 2. LINEAR INTERPOLATION BETWEEN VALUES OF TRIBUTARY AREA IS PERMISSIBLE. 3. PLUS AND MINUS SIGNS SIGNIFY PRESSURE TOWARD AND AWAY FROM THE EXTERIOR SURFACE, RESPECTIVELY.

1.5 WIND LOADS (CONT)

1.5.4 DESIGN WIND PRESSURE - ROOF COMPONENTS AND CLADDING ROOF COMPONENTS & THEIR ATTACHMENTS SHALL BE DESIGNED FOR THE PRESSURES SHOWN IN THE ADJACENT DIAGRAM & TABLE BELOW:



ROOF MEAN HEIGHT

LOCATION	GROS	а				
	1	2	3			
MAIN BUILDING				(mm)		
$AREA = 1 m^2$	-866	-1513	-1513	900		
$AREA = 2 m^2$	-866	-1513	-1513	900		
$AREA = 5 m^2$	-866	-1513	-1513	900		
0	200	4543	4545			

- 1. DESIGN WIND PRESSURES ABOVE REPRESENT THE NET PRESSURE (SUM OF INTERNAL AND EXTERNAL PRESSURE) APPLIED NORMAL TO ALL SURFACES. 2. LINEAR INTERPOLATION BETWEEN VALUES OF TRIBUTARY AREA IS PERMISSIBLE. 3. PLUS AND MINUS SIGNS SIGNIFY PRESSURE TOWARD AND AWAY FROM THE
- EXTERIOR SURFACE. RESPECTIVELY.

 $AREA = 10 \text{ m}^2$

CONCRETE COLUMN SCHEDULE SIZE REINFORCING MARK (LxW) VERT BARS TIES #16 @ 100 (CORE OF EXT & CORNER COLUMNS) (8)-#25C1 400x400 #13 @ 150 (CORE OF INTERIOR COLUMNS) #13 @ 200 (OTHER)

- 1. DIMENSIONS NOTED ARE MILLIMETERS (mm) UON. 2. SECOND STORY COLUMNS ARE SAME AS FIRST STORY COLUMNS.
- 3. CORE INDICATES THE AREA OF COLUMN & BEAM INTERSECTION
- 4. TIE INDICATES PERIMETER & CROSS TIE COMBINED

2.0 FOUNDATION DESIGN CRITERIA (TO BE CONFIRMED BY THE CONTRACTOR)

THE GEOTECHNICAL ANALYSIS FOR THIS PROJECT IS THE RESPONSIBILITY OF THE CONTRACTOR AWARDED THE WORK. DESIGN VALUES USED IN THE STRUCTURAL ANALYSIS OF THE BUILDINGS HEREIN INDICATED HAVE BEEN ASSUMED AND SHALL BE CONFIRMED AND VERIFIED AS PART OF THE GEOTECHNICAL INVESTIGATION. VALUES WHICH DO NOT MEET THE REQUIREMENTS INDICATED BELOW SHALL BE IMMEDIATELY BROUGHT TO THE ATTENTION OF THE CONTRACTING OFFICER FOR CONSIDERATION AND DETERMINATION ON THE NEXT APPROPRIATE COURSE OF ACTION.

2.1.1 SOIL DESIGN PARAMETERS

NET ALLOWABLE SOIL BEARING CAPACITY	96.0 KPa
UNIT WEIGHT OF SOIL (moist)	1800 Kg/m ³
COEFF ACTIVE EARTH PRESSURE (Kpa)	0.30
COEFF PASSIVE EARTH PRESSURE (Kpp)	3.33
COEFF AT-REST EARTH PRESSURE (Kpr)	.55
COEFF OF SOIL FRICTION	.35
SUBGRADE MODULUS	4120 g/m ³

MINIMUM BEARING DEPTH BELOW GRADE

SEISMIC SITE CLASS (based on in—situ soil)

CONCRETE COVER SCHEDULE

800mm

MINIMUM CONCRETE COVER PROTECTION FOR REINFORCEMENT BARS SHALL BE AS FOLLOWS: (SEE ACI 318M-05, SECTION 7.7 FOR CONDITIONS NOT NOTED) DIMENSIONS FOR BAR PLACEMENT GIVEN IN SECTIONS AND DETAILS SHALL SUPERSEDE MINIMUM COVER REQUIREMENTS GIVEN HERE. DIMENSIONS ARE IN mm.

FOOTINGS (EARTH FORMED)	70
COLUMNS / PIERS (TO TIES)	40
GRADE BEAMS OR SLAB TURNED DOWN EDGES:	
TOP BOTTOM (EARTH FORMED) SIDES (EARTH FORMED) SIDES (BOARD FORMED) #16 BAR & SMALLER #19 THRU #36 BAR	40 70 70 40 50
ELEVATED BEAMS & SLABS:	
BEAM TIES & STIRRUPS (NOT EXPOSED TO WEATHER)	40

40
50
20
50
40
25
20

UTILITY TUNNEL WALLS, RETAINING WALLS AND SHEAR WALLS, (NO SURFACES SHALL BE EARTH FORMED) EARTH SIDE AND FRONT SIDE (EXPOSED TO WEATHER):

SLABS-ON-GRADE (EXPOSURE TO WEATHER) FROM TOP

#16 BAR AND SMALLER

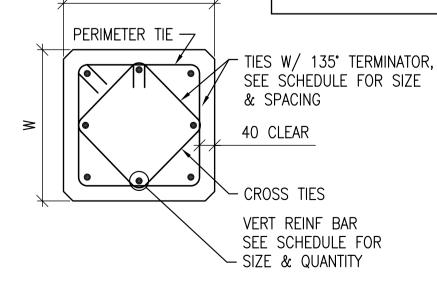
CONCRETE PROTECTION SPECIFIED.

50 #19 THRU #36 BAR PROVIDE STANDARD BAR CHAIRS AND SPACERS AS REQUIRED TO MAINTAIN

MASONRY CONCRETE LINTEL SCHEDULE

OPENING TYPE OR SIZE, BEAM LOCATION OR	MAX SPAN	BEAM DEPTH	MAIN	REINFORC	ING	CLIEAD DEINE CTIDDUDC
TYPÉ	(mm)	(mm)	TOP	воттом	OTHER	SHEAR REINF STIRRUPS
INT WALL OPENING, NON-BEARING	1800	400		(2)-#13		
INT WALL OPENING, NON-BEARING	900	200		(2)-#13		
		•	-	-		-

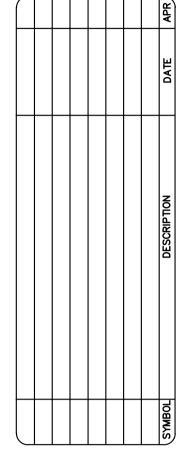
- 1. STRUCTURAL DRAWINGS DO NOT INDICATE ALL OPENINGS IN MASONRY WALLS. VERIFY NUMBER, SIZE AND LOCATION OF ALL OPENINGS IN MASONRY WALLS FROM ARCHITECTURAL SHEETS AND APPROVED PLUMBING, MECHANICAL, AND ELECTRICAL SHOP DRAWINGS.
- 2. PROVIDE 200mm BEARING EA END FOR 200mm DEEP MASONRY LINTEL PROVIDE 400mm
- BEARING EA END FOR 400mm DEEP CIPL.
- 3. FOR HEAD DETAILS REFER TO ARCHITECTURAL SHEETS.
- 4. REINFORCING SHALL BE ASTM A615M, GRADE 420. CONCRETE FOR CAST-IN-PLACE BEAMS SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 28 MPa AT 28 DAYS.
- 5. CONTRACTOR SHALL SUBMIT FOR APPROVAL SHOP DRAWINGS AND SCHEDULES SHOWING SIZE, DETAILS, LOCATIONS, ETC FOR ALL CAST-IN-PLACE BEAMS IN CMU WALLS.







District

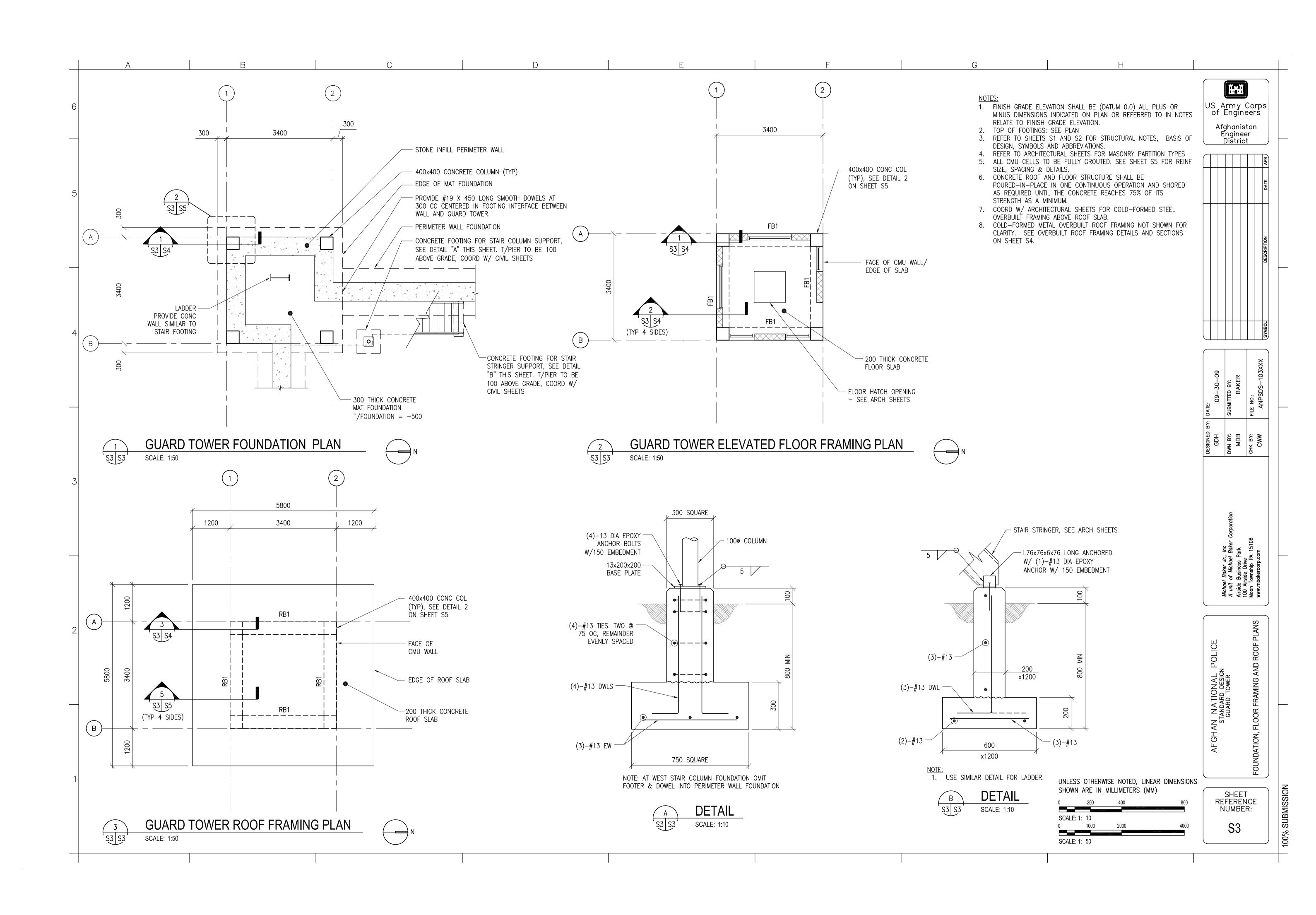


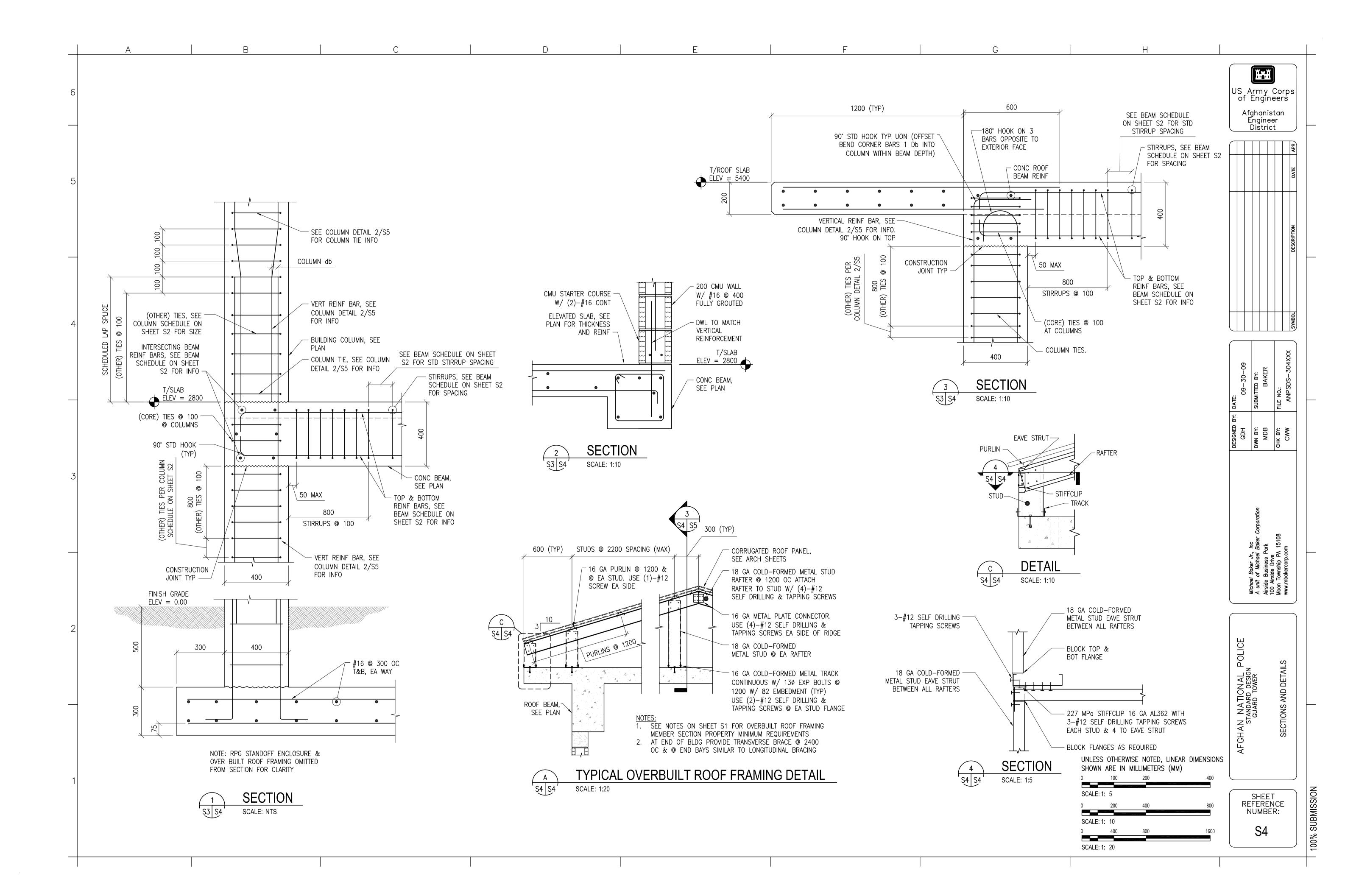
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DATE:	60-30-60	SUBMITTED BY:	BAKER	FILE NO.:	ANPSDS-002XXX
DESIGNED BY: DATE:	НОЭ	DWN BY:	MDB	CHK BY:	CWW
	Wichael Raber Ir Inc	A unit of Michael Baker Corporation	Airside Business Park	Moon Township PA 15108	www.mbakercorp.com

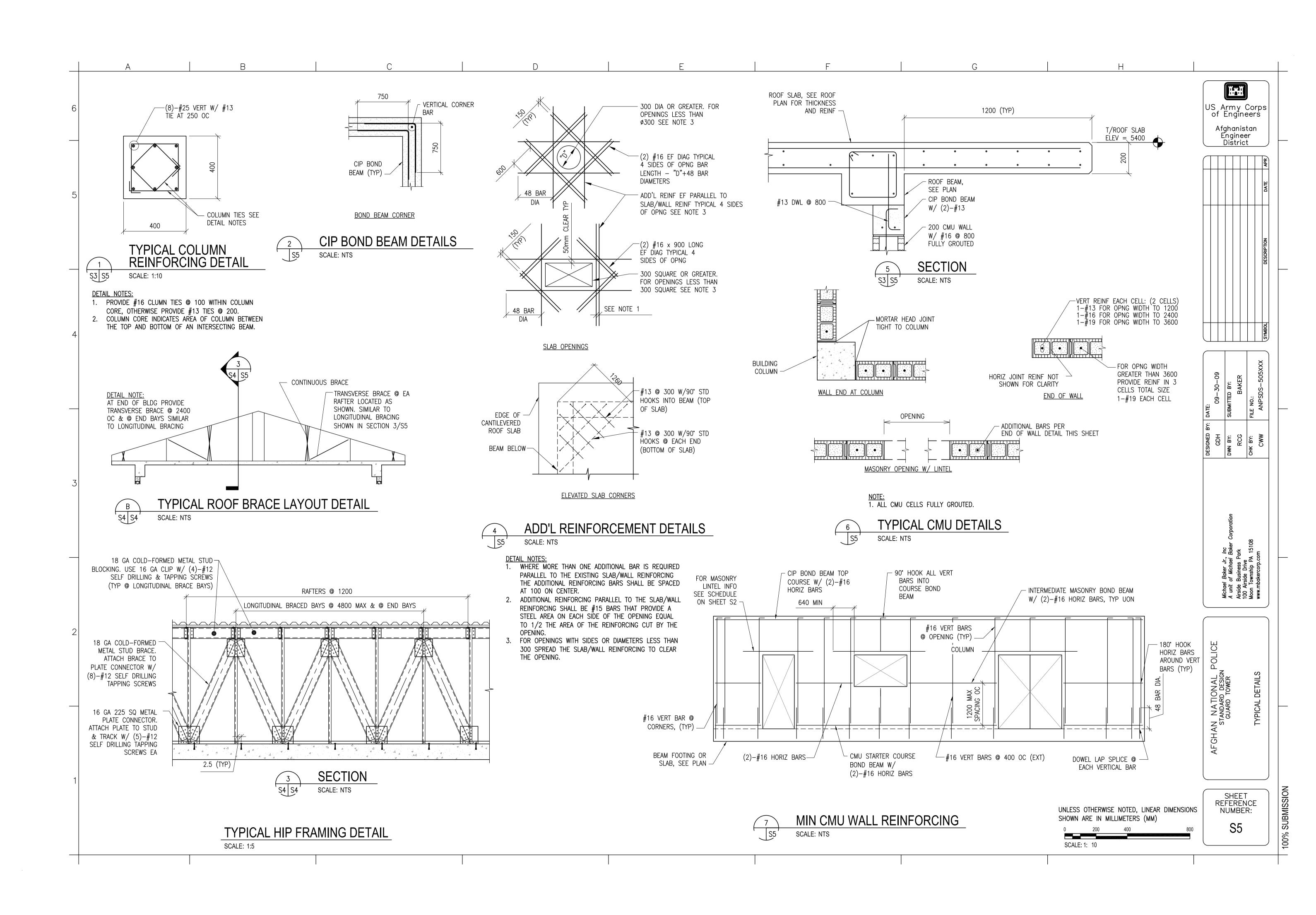
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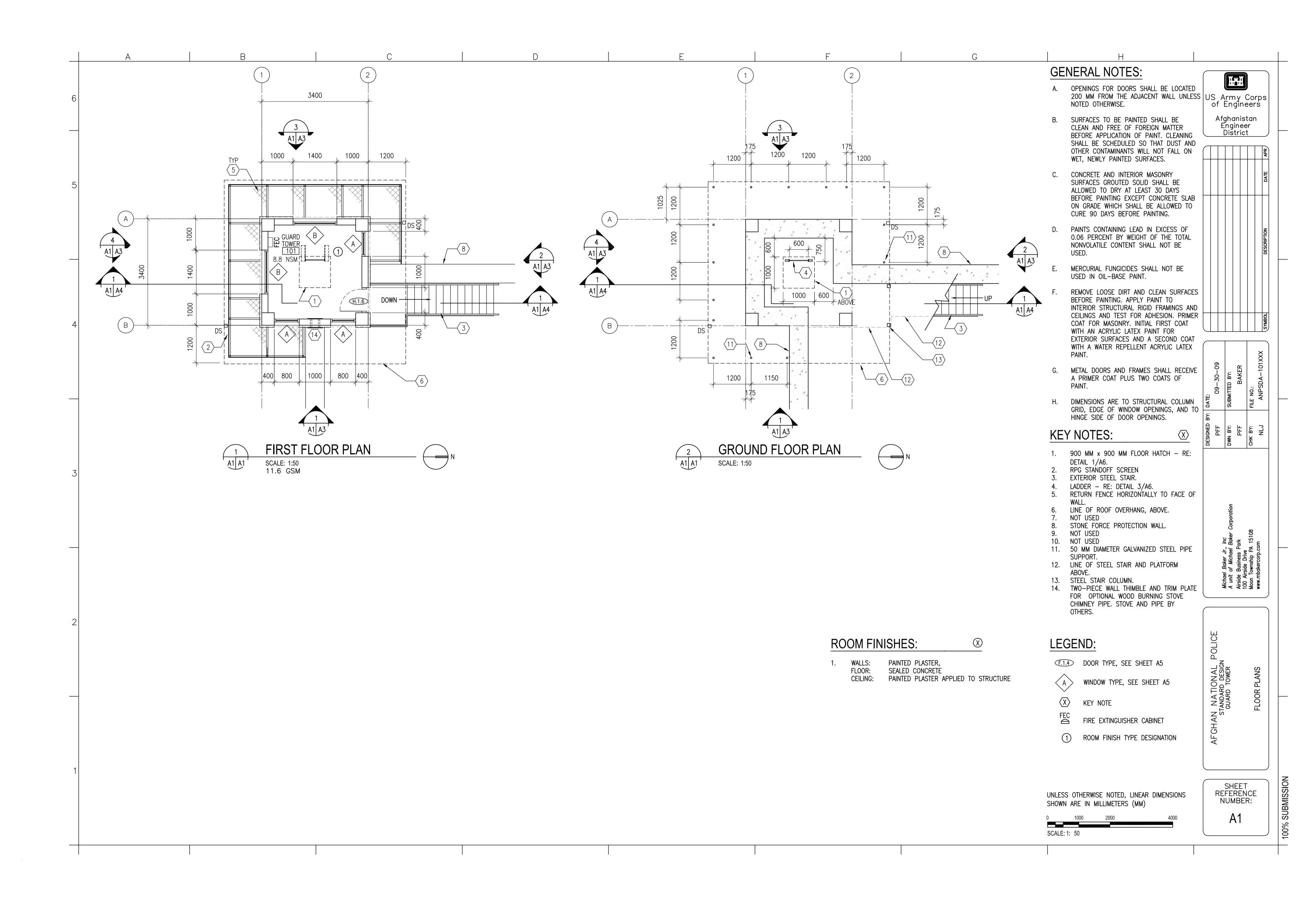


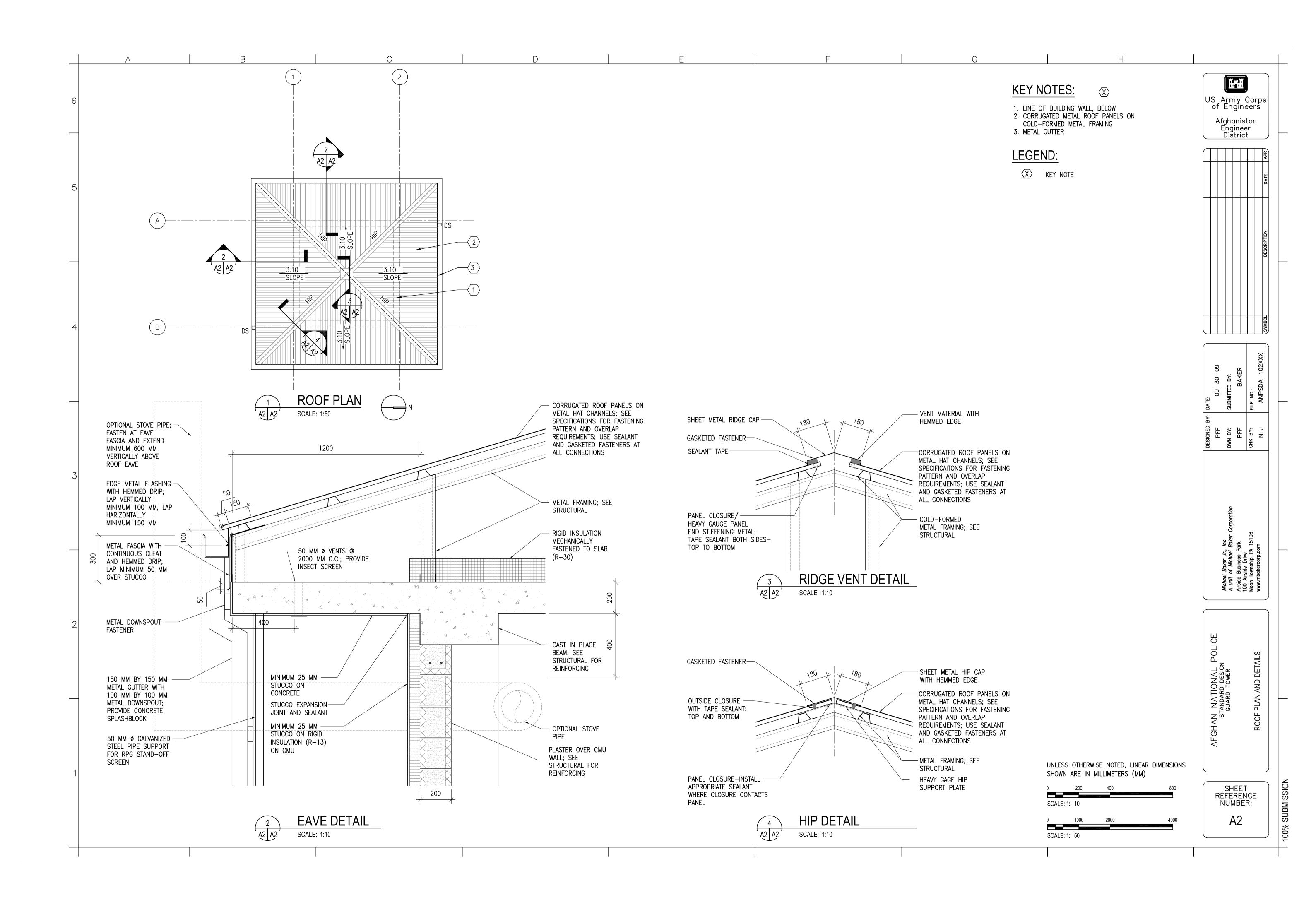
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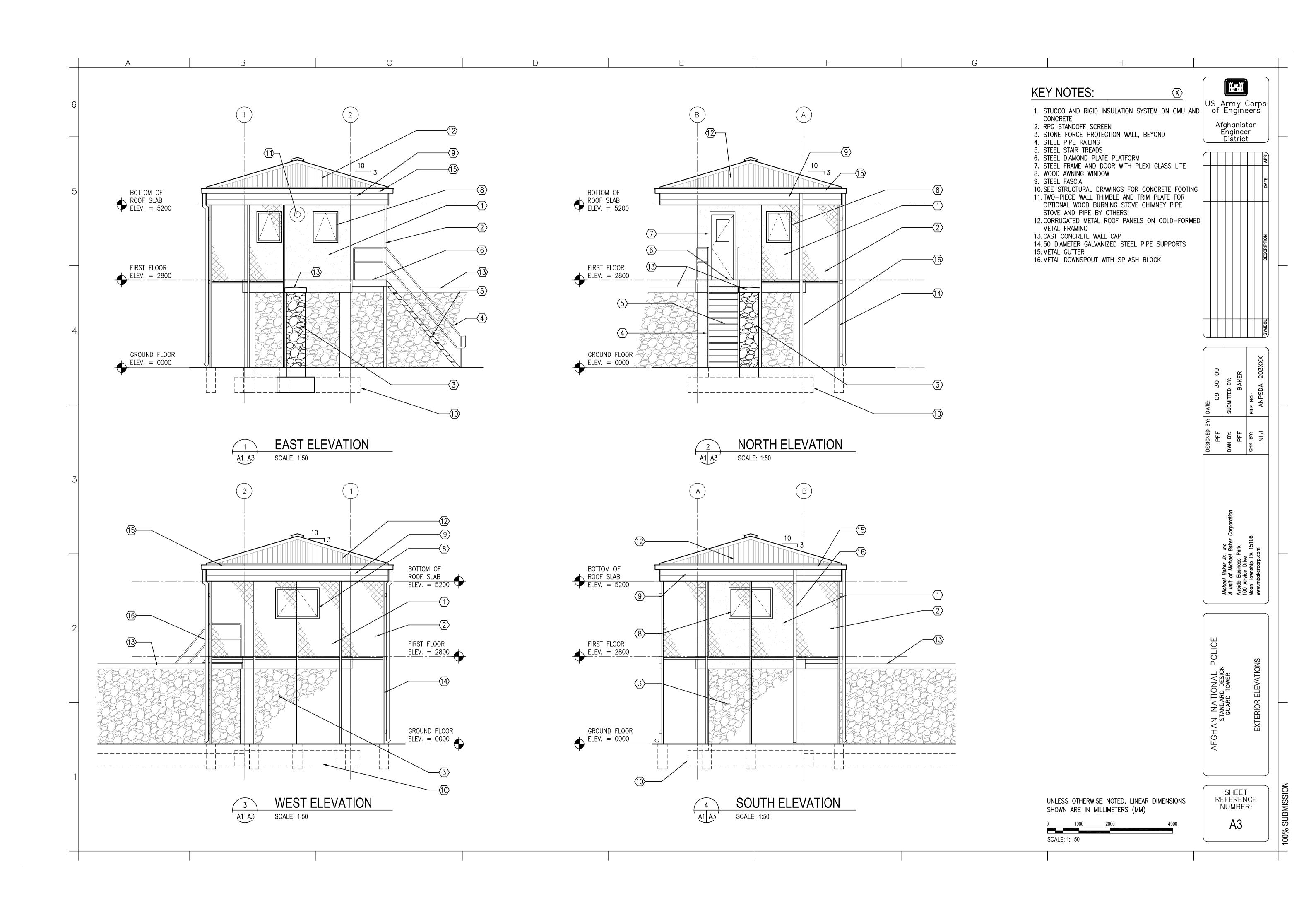


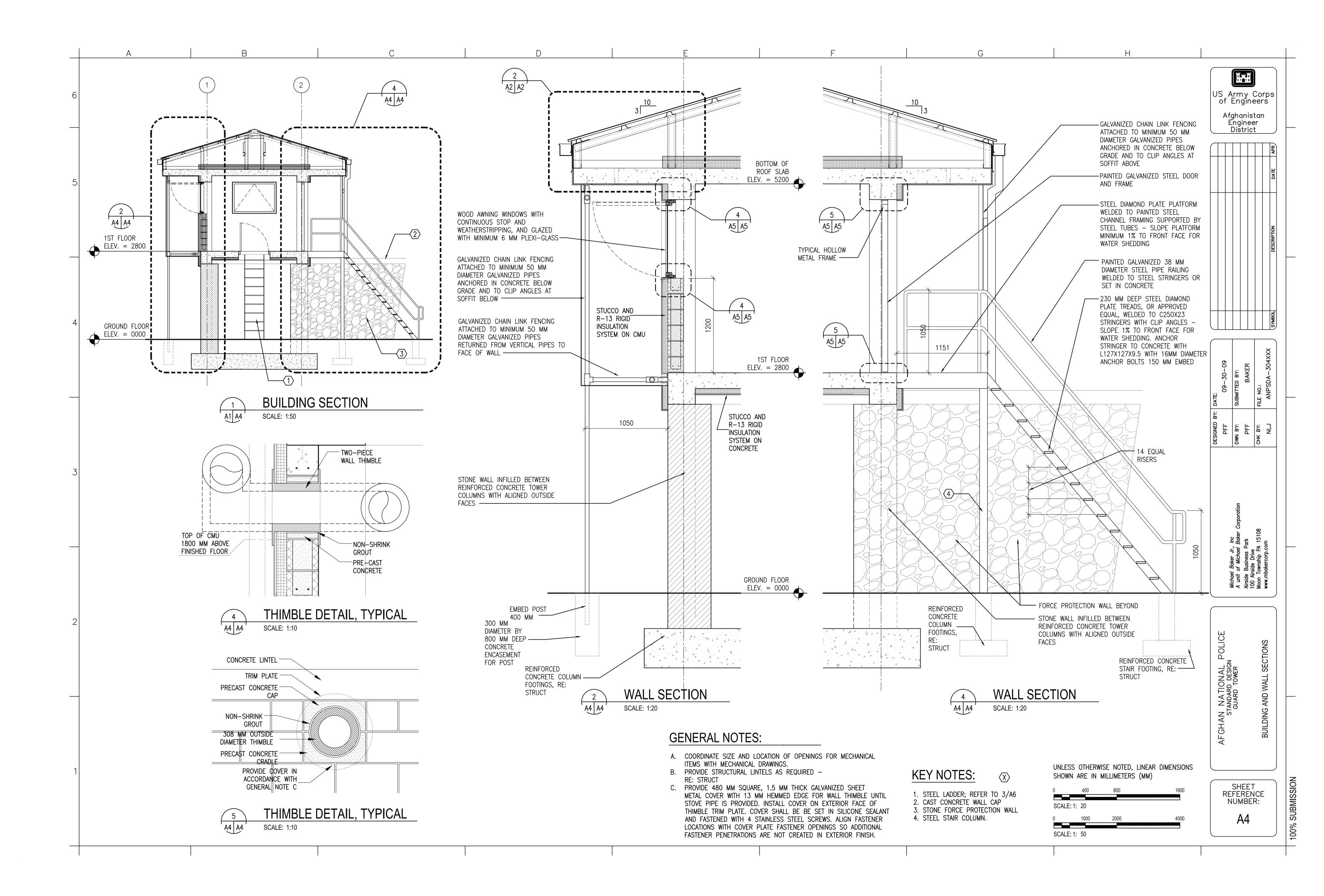


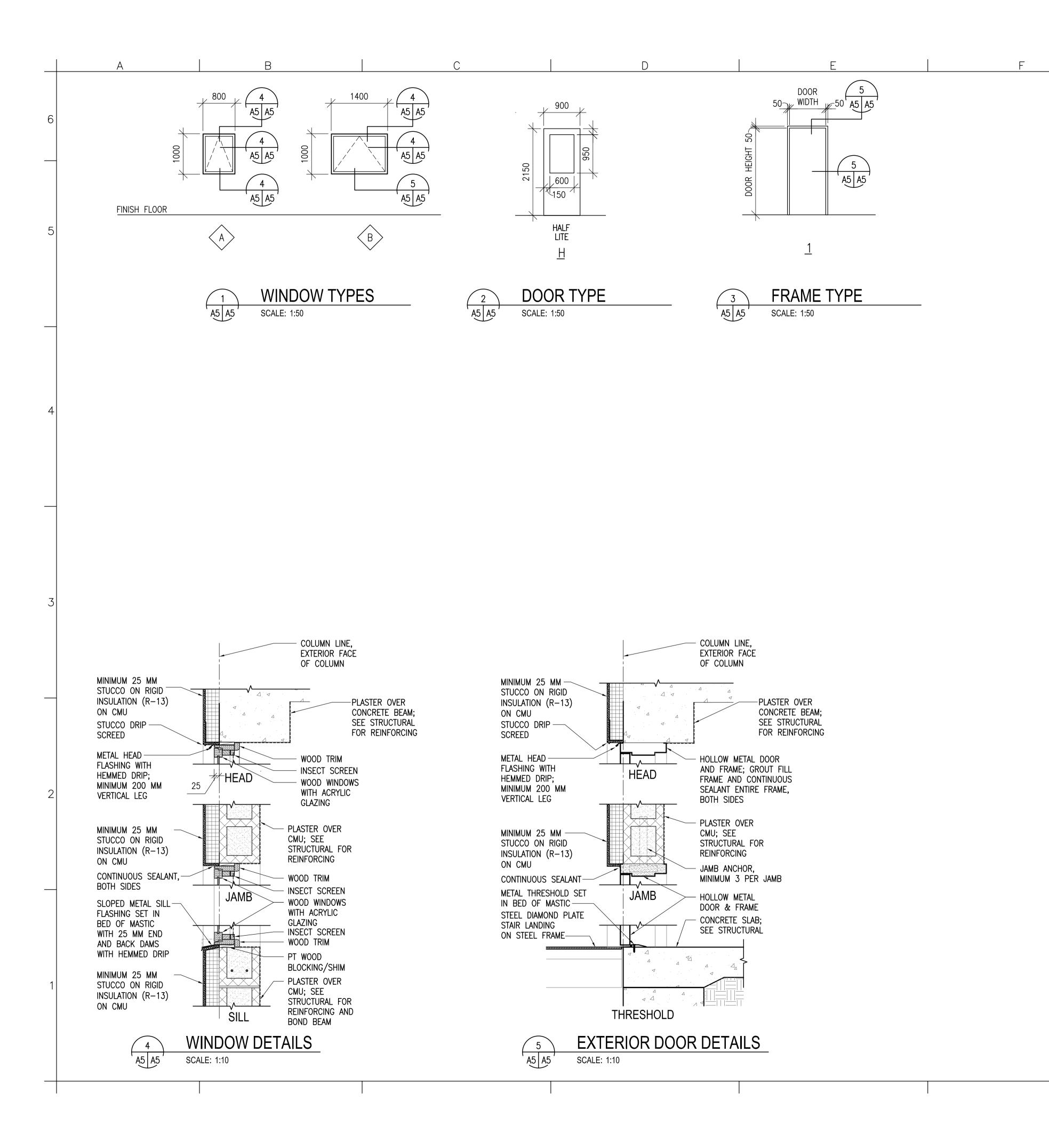












WINDOW TYPES NOTES:

- 1. ALL EXTERIOR WINDOWS SHALL BE WOOD WITH INSECT SCREENS. WINDOWS SHALL BE COMMERCIAL GRADE.
- 2. GLAZING SHALL BE ACRYLIC SHEET.

DOOR TYPES NOTES:

- 1. EXTERIOR METAL DOORS AND FRAME COLORS SHALL MATCH ADJACENT WALL COLORS AS SELECTED BY THE CONTRACTING OFFICER.
- 2. HARDWARE SHALL BE HEAVY DUTY, COMMERCIAL GRADE, STAINLESS STEEL WITH A MATTE FINISH.
- 3. FRAMES, EXCEPT FIRE—RATED FRAMES, SHALL BE MOUNTED AND ADJUSTED IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS. FRAMES SHALL BE FASTENED WITH MINIMUM OF THREE FASTENING POINTS PER SIDE AT REGULAR INTERVALS.
- 4. DIMENSIONS SHOWN ON DOOR SCHEDULE ARE BASED UPON MODULAR MASONRY (OR ROUGH OPENING), HEIGHT OF 2200mm FOR STANDARD PERSONNEL DOORS. CONTRACTOR SHALL COORDINATE WITH DOOR SUPPLIER TO ENSURE THAT DIMENSIONS OF DOORS AND FRAMES PROVIDED ARE COMPATIBLE WITH DOOR OPENING DIMENSIONS.



HARDWARE TYPES:

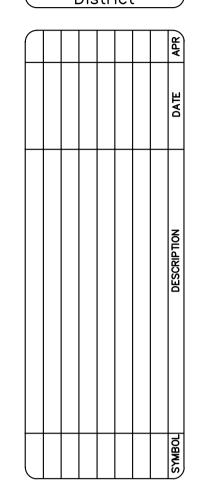
HW-6 1-1/2 PR HINGES

1 EA LOCKSET, FO4 ENTRY LOCK W/LEVERS. GRADE 1

1 EA DOOR CLOSER, CO2061,LOW RESISTANCE

1 EA DOOR CLOSER, COZO 1 EA THRESHOLD J32130

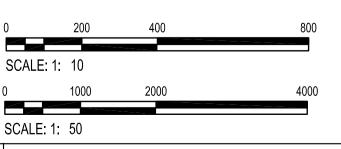




	DESIGNED BY: DATE:	DATE:
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er Corporation	DWN BY:	SUBMITTED BY:
Airside Business Park	PFF	BAKER
A 15108	CHK BY:	FILE NO.:
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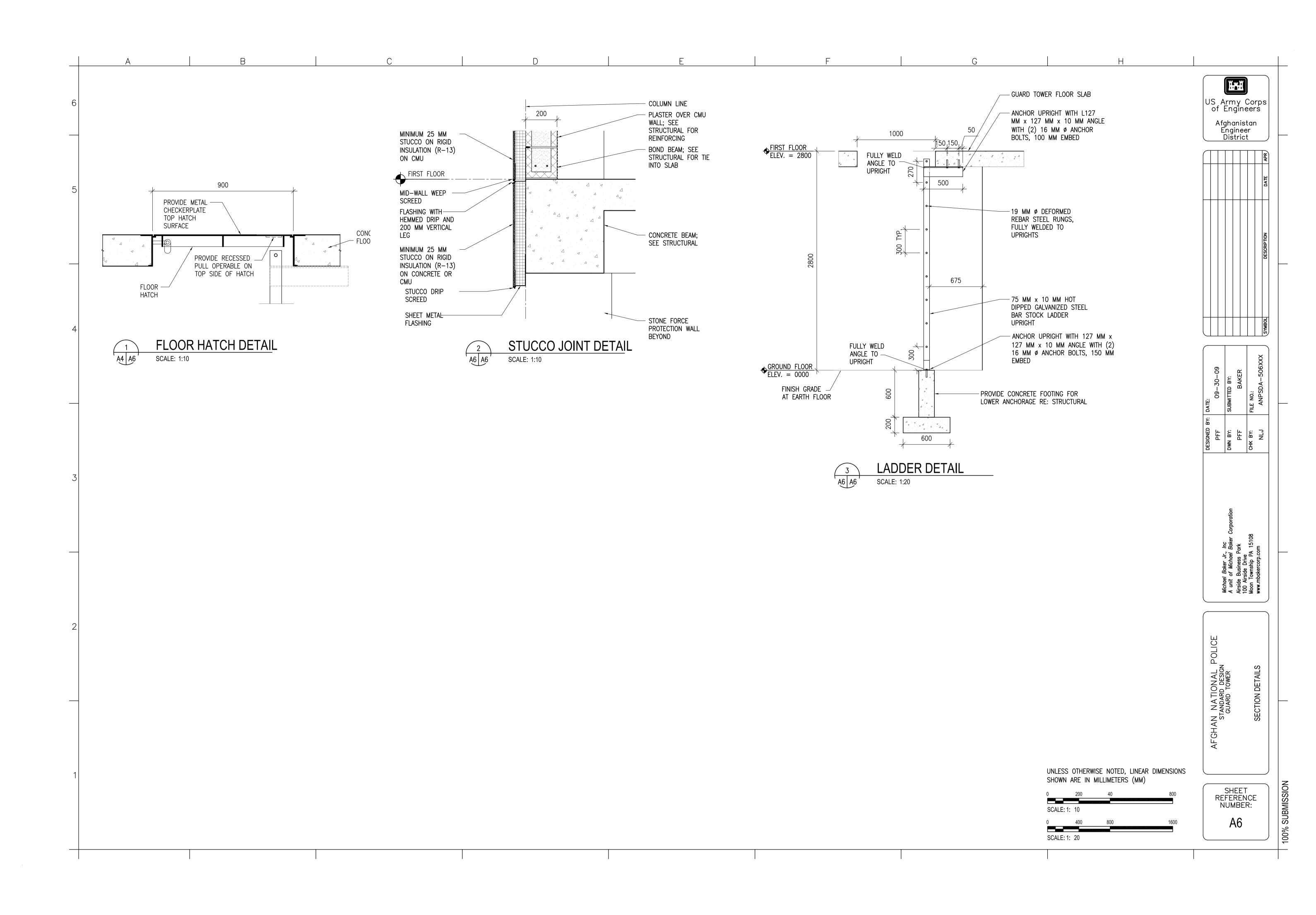


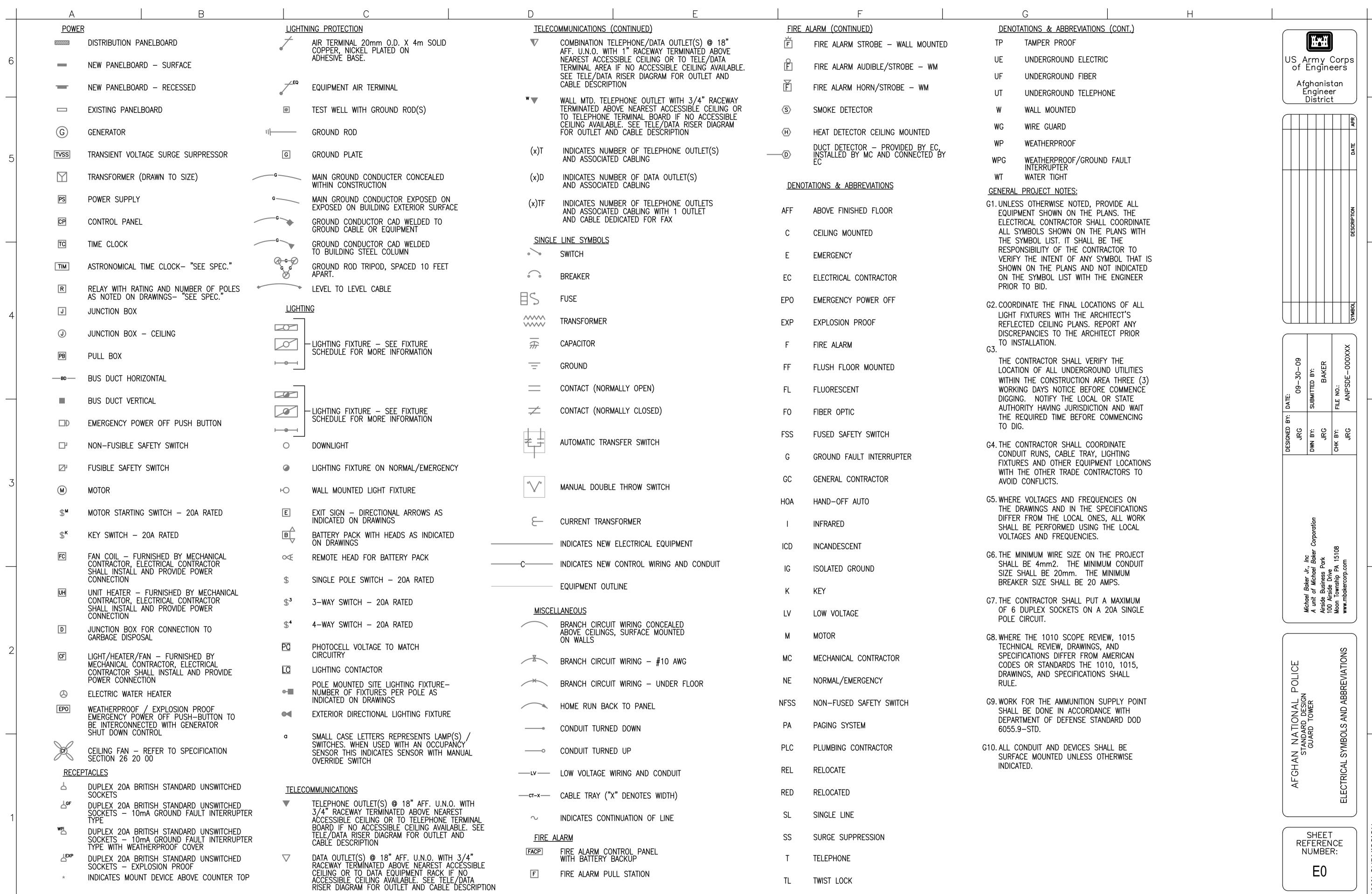
UNLESS OTHERWISE NOTED, LINEAR DIMENSIONS SHOWN ARE IN MILLIMETERS (MM)

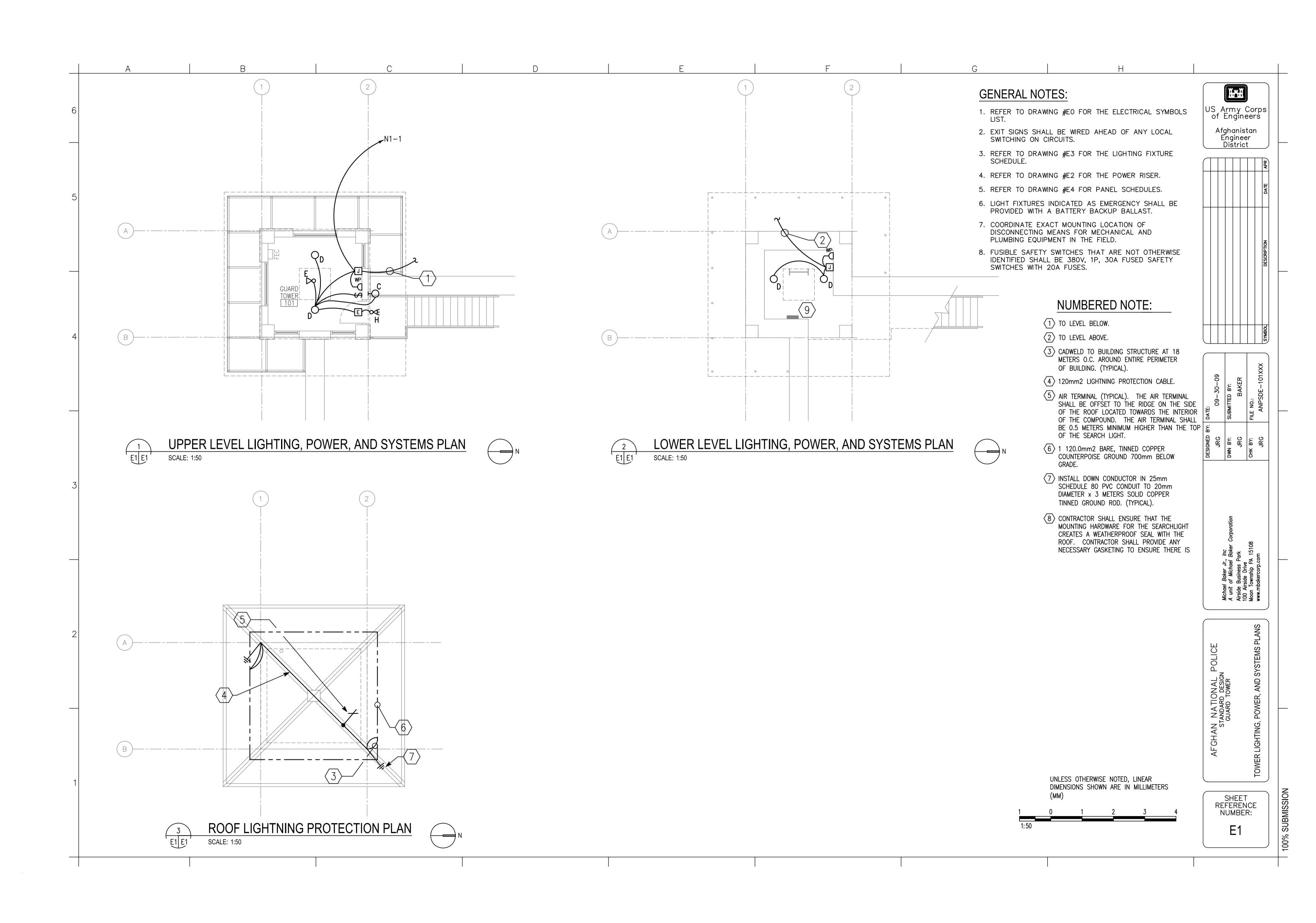


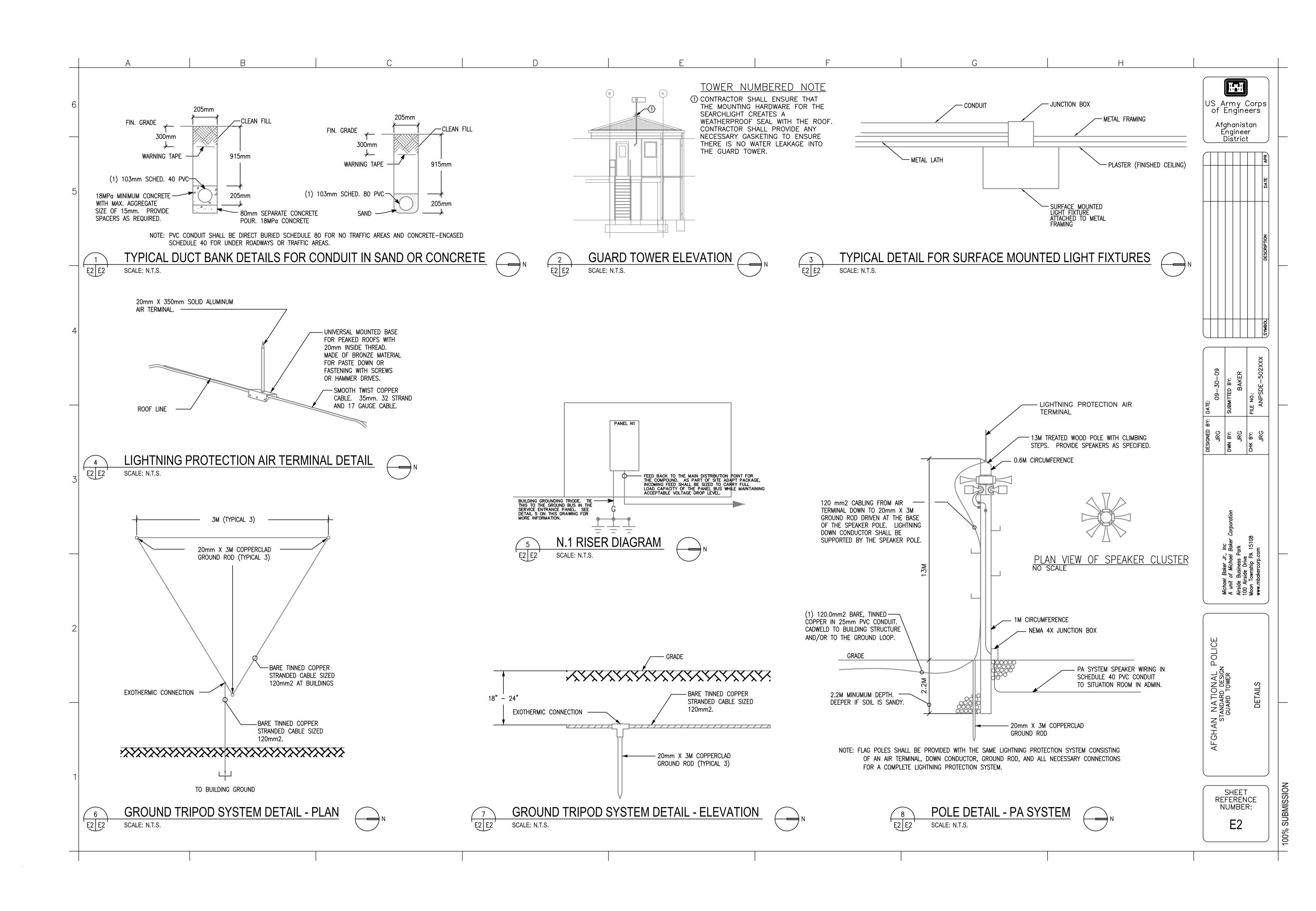
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100% SUBMISSION









FIXTURE MARK 'C'



INCANDESCENT ONE PIECE WITH APPROVED LENS, STABILIZED HIGH IMPACT POLY CARBONATE

FIXTURE MARK 'D'



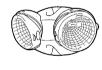
SURFACE MOUNTED, SINGLE LAMP 220-240V INCANDESCENT FIXTURE RATED FOR 100W LAMPS. GLASS GLOBE SHALL BE VAPOR TIGHT JELLY JAR TYPE WITH WIRE GUARD. NOMINAL DIMENSIONS ARE 111MMx251MM

FIXTURE MARK 'E'



SEALED BEAM SPOTLIGHT MOUNTED TO GUARD TOWER ROOF WITH 1000W HALOGEN NARROW SPOT PAR 56 LAMP. FIXTURE SHALL BE PROVIDED WITH TWIST LEVER CONTROL, MIN. 254MM DIAMETER LIGHT HEAD WORKING RANGE OF ½ MILE, RATED FOR OUTDOOR ROUGH SERVICE WITH ANTI—SHOCK LAMP HOLDER

FIXTURE MARK 'H'



REMOTE HEAD EXTERIOR LIGHT HEAD POWERED FROM EXIT SIGN BATTERY—12V DOUBLE HEAD CORROSION RESISTANT WITH UL34 WEATHERPROOF CONSTRUCTION

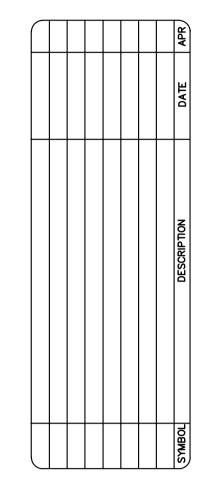
FIXTURE MARK 'E'



UNIVERSAL MOUNT ENGINEER GRADE
THERMOPLASTIC HOUSING EXIT SIGN WITH
LED LAMPS, RED LETTERS 6" IN HEIGHT
WITH ARROWS AS INDICATED, WITH 12V
CADMIUM BATTERY

		LIGHTING FI	XTURE SCI	HEDULE	
FIXTURE MARK	STYLE NUMBER AND TYPE	NUMBER AND TYPE OF LAMPS	VOLTAGE	MOUNTING	NOTES
С	INCANDESCENT ONE PIECE W/ APPROVED LENS STABILIZED HIGH IMPACT POLY CARBONATE.	(1) A19 - 100W INCANDESCENT	220V - 1ø 50HZ	WALL MOUNTED ABOVE EXTERIOR DOORS	
D	SURFACE MOUNTED SINGLE LAMP 220-240V INCANDESCENT FIXTURE RATED FOR 100W LAMPS. GLASS GLOBE SHALL BE VAPOR TIGHT JELLY JAR WITH WIRE GUARD. 111MM-251MM	(1) A21 - 100W COATED INCANDESCENT	220V – 1ø 50HZ	SURFACE MOUNTED FROM ROOF STRUCTURE	MEETS INTERNATIONAL STANDARDS WITH ENCLOSED AND GASKETED RED LENS.
E	SEALED BEAM SPOTLIGHT MOUNTED TO GUARD TOWER ROOF WITH 1000W HALOGEN NARROW SPOT LAMP	(1) PAR 56 1000W HALOGEN NARROW SPOT LAMP	220V - 1ø 50HZ	MOUNTED TO GUARD TOWER ROOF	SEE FIXTURE/TOWER DETAIL FOR SPOTLIGHT MOUNTING.
Н	REMOTE HEAD EXTERIOR LIGHT HEAD POWERED FROM EXIT SIGN BATTERY— 12V DOUBLE HEAD CORROSION RESISTANT WITH UL34 WEATHERPROOF CONSTRUCTION	(2) 12W/12V HALOGEN LAMP	12V – 1ø 50HZ	EXTERIOR WALL MOUNTED AT TOP OF DOOR HEIGHT	
E	UNIVERSAL MOUNT ENGINEER GRADE THERMOPLASTIC HOUSING EXIT SIGN WITH LED LAMPS, RED LETTERS 6" IN HEIGHT WITH ARROWS AS INDICATED, WITH 12V CADMIUM BATTERY WITH REMOTE HEAD CAPABILITY	LED LAMPS	220V — 1ø 50HZ	UNIVERSAL MOUNTING	





	DESIGNED BY: DATE:	DATE:
Dalor is	JRG	09-30-09
Micriael Duker VI., III. A unit of Michael Baker Corporation	DWN BY:	SUBMITTED BY:
Airside Business Park 100 Airside Drive	JRG	BAKER
ownship PA 15108	CHK BY:	FILE NO.:
www.mbakercorp.com	JRG	ANPSDE-603XXX

AFGHAN NATIONAL POLICE
STANDARD DESIGN
GUARD TOWER
LIGHT FIXTURE SCHEDULE

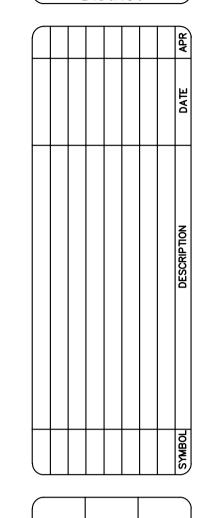
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PANELBOARD N1 SURFACE MOUNTED ASYM. A.I.C. MIN. AMP. MAIN LUGS (OR) 100 AMP. MAIN BREAKER W/ 100 AMP. TRIP CIRCUIT BREAKER TYPE 380/220 VOLTS 3 PHASE 4 WIRE 50 HZ 100 AMP. BUS TRIP SS WIRE GND CONDUIT MM 2 MM2 MM CONDUIT GND WIRE SI TRIP AMPS SI LOAD-V.A. LOAD-V.A. LOAD SERVED LOAD SERVED AØ BØ CØ AØ BØ CØ 1 20 2 1 20 1 4.0 4.0 20 LIGHTING AND RECEPTACLES 2.0 SPARE 1 20 4 3 20 1 SPARE SPARE 1 20 6 5 20 1 SPARE SPARE 7 20 1 1 20 8 SPARE SPARE 1 20 10 9 20 1 SPARE SPARE 1 20 12 SPARE SPARE 11 20 1 2.0 0.0 0.0 0.0 0.0 0.0 TOTAL CONN. LOAD PER PHASE (KVA): AØ 2.0 BØ 0.0 CØ 0.0

* MAIN BREAKER SHALL BE 3P EARTH GROUND TYPE

TOTAL CONN. LOAD 2.0 KVA. 70 % DEMAND = ESTIMATED DEMAND LOAD 1.4 SUPPLIED FROM MAIN SWITCHGEAR DISTRIBUTION

US Army Corps of Engineers Afghanistan Engineer District



		i i
	JRG	09-30-0
Michael Baker Jr., Inc A unit of Michael Baker Corporation	DWN BY:	SUBMITTED BY:
Airside Business Park 100 Airside Drive	JRG	BAKER
Moon Township PA 15108	CHK BY:	FILE NO.:
www.mbakercorp.com	JRG	ANPSDE-60

AFGHAN NATIONAL POLICE STANDARD DESIGN GUARD TOWER

SHEET REFERENCE NUMBER:

ICE R:

AFGHAN NATIONAL POLICE STANDARD BUILDING DESIGNS

SITE DETAILS

SHEET	INDEX
G1	COVER SHEE
C1	CIVIL DETAILS
C2	CIVIL DETAILS
C3	CIVIL DETAILS
C4	CIVIL DETAILS
C5	CIVIL DETAILS
C6	CIVIL DETAILS
C7	

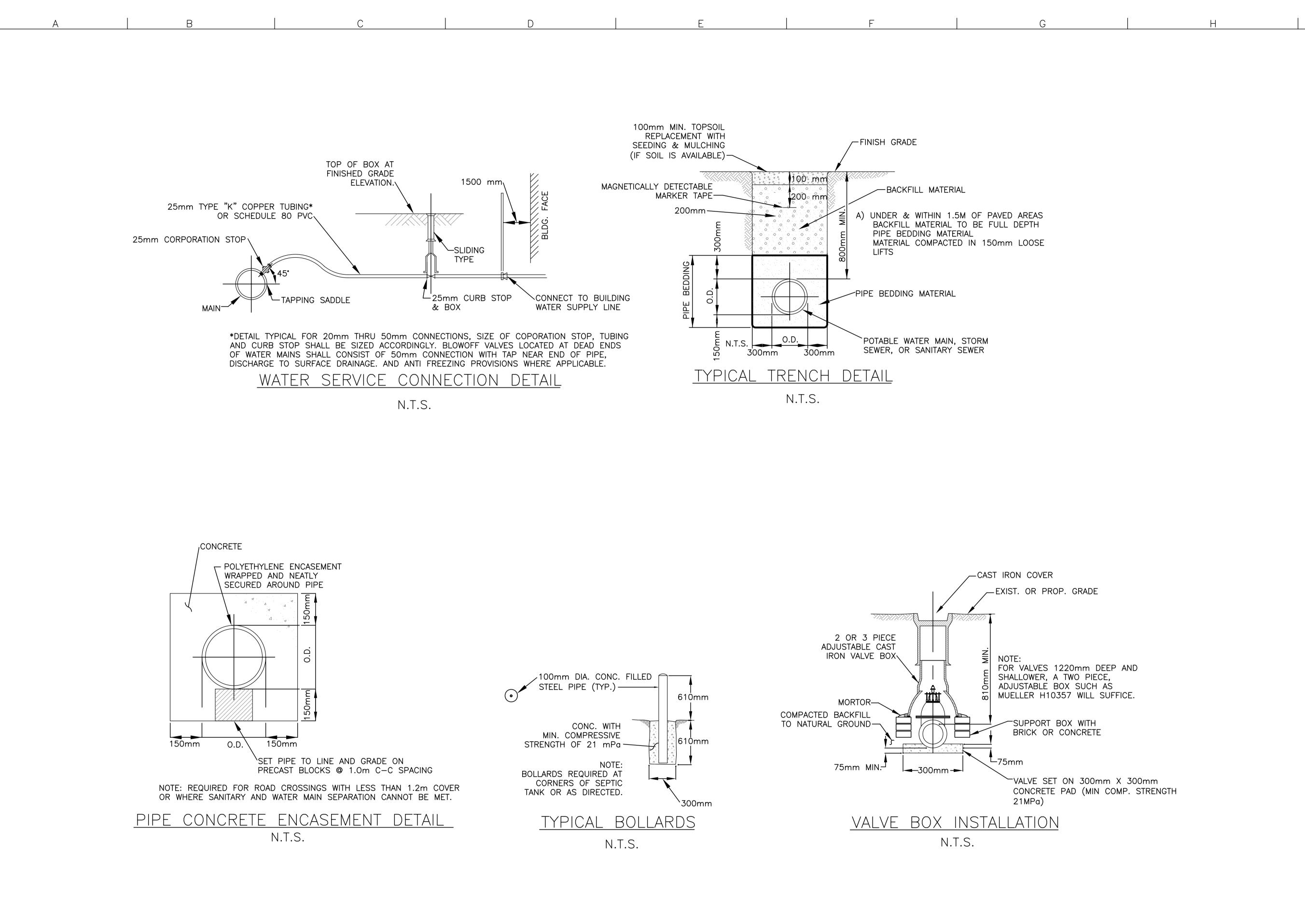


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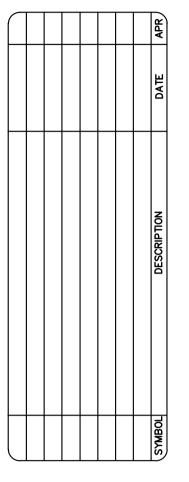
	DESIGNED BY: DATE:	DATE:
Hishael Bokor Is Inc	Sar	09-30-09
Michael Baker Or., III. A unit of Michael Baker Corporation	DWN BY:	SUBMITTED BY:
Airside Business Park 100 Airside Drive	SON	BAKER
Moon Township PA 15108	CHK BY:	FILE NO.:
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AFGHAN NATIONAL POLICE STANDARD DESIGN SITE DETAILS COVER SHEET

> SHEET REFERENCE NUMBER:



HAH US Army Corps of Engineers Afghanistan Engineer Dištrict



DATE:	60-30-60	SUBMITTED BY:	BAKER	FILE NO.:	ANPSDC-501XXX
DESIGNED BY: DATE:	BAKER	DWN BY:	Sar	CHK BY:	GPH

AN NATIONAL POLICE STANDARD DESIGN SITE DETAILS

SHEET REFERENCE NUMBER:

